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Foundation Engineering for Expansive Soils

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Preface

The practice of foundation engineering was first developed to address problems associated with settlement due to saturated soils that were prevalent in areas with soft coastal and deltaic deposits. As population and business centers moved into areas with more arid climates, problems with other types of soils became evident. Some soils that were capable of supporting a load in a natural unsaturated state were observed to either expand or collapse when wetted. These soils did not conform to the classical theories of soil mechanics and foundation engineering, and more research began to focus on the behavior of unsaturated soils.

Within the general category of unsaturated soils, the expansive soils posed the greatest problems, and created the most financial burden. In response to major infrastructure development in the late 1950s and 1960s there was an upswing in research regarding the identification of expansive soils and factors influencing their behavior. Engineers became more cognizant of the need for special attention to the unique nature of expansive soils.

The general curricula taught at universities did not specifically address the design of foundations for these soils, and engineers did not become aware of expansive soils unless they began to practice in areas where those soils existed. Therefore, the practice of foundation engineering for expansive soils developed around experience and empirical methods.

Few books have been written specifically on the subject of design of foundations on expansive soils. Fu Hua Chen

wrote a book entitled *Foundations on Expansive Soils* that was published in 1975. A second edition of that book was published in 1988. Those books were based to a large extent on Mr. Chen's personal experiences along the Front Range of Colorado. The Department of the Army published a technical manual in 1983 titled, *Foundations for Expansive Soils*. That manual served as the basis for the design of structures on military bases, and was available to the civilian engineering community as well.

At about that same time the US National Science Foundation funded a research project at Colorado State University (CSU) dealing with expansive soils. The scope of that project included a survey of the practices followed by engineers throughout the United States and Canada, as well as individuals from other countries. On the basis of that survey, and research that had been conducted by that time, Nelson and Miller (1992) published a book entitled *Expansive Soils: Problems and Practices in Foundation and Pavement Engineering*.

In 1993 Fredlund and Rahardjo published their text, *Soil Mechanics for Unsaturated Soil*. That book extended the framework of classical soil mechanics to incorporate soil suction as an independent stress state variable, and provided the rigor needed for a theoretical understanding of unsaturated soils. A part of that book was devoted to the mechanics of expansive soil.

In the 20 years since the publication of Nelson and Miller (1992), the authors of this book have worked together and have performed hundreds of forensic investigations on expansive soils. In the course of that work, many new ideas have emerged, additional research has been conducted, and methods of analyses were developed that have been applied to foundation design. This book reflects the authors' experiences over the period since the book by

Nelson and Miller was written. It incorporates a broader scope of analysis and a greater degree of rigor than the earlier work.

In a presentation at the 18th International Conference on Soil Mechanics and Geotechnical Engineering in which he introduced his most recent book, *Unsaturated Soil Mechanics in Engineering Practice*, Dr. Fredlund noted the need for practitioners to continue to publish works that will extend the application of the concepts of unsaturated soil mechanics to the solution of practical geotechnical engineering problems. It is believed that this book responds to that call and will provide a sound basis on which to establish a practice of foundation engineering for expansive soils.

Many people have contributed to the completion of this book, most notably Ms. Georgia A. Doyle. She has read the entire manuscript, provided necessary and valuable editing and coordination, and queried the authors where material was not clear. Many valuable comments were received from Dr. Donald D. Runnells after his review of chapter 2, and Dr. Anand J. Puppala after his review of chapter 10. Professor Erik. G. Thompson developed the FEM analysis for the APEX program presented in chapter 12.

In addition, many current and former staff of the authors' company, Engineering Analytics, Inc., have contributed in one way or another. Special recognition goes to Kristle Beaudet, Todd Bloch, Denise Garcia, Debbie Hernbloom, Jong Beom Kang, Lauren Meyer, Ronald Pacella, and Rob Schaut. Their help along the way is much appreciated.

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List of Symbols

clay activity $A_{\rm c}$ width of footing \boldsymbol{R} slope of matric suction vs water content curve Bcohesion of soil \boldsymbol{C} Cmolar concentration $C_{\rm c}$ compression index "Department of Army" heave index C_{DA} $C_{\rm H}$ heave index suction compression index $C_{\rm h}$ $C_{\rm m}$ matric suction index C_{p} peak cohesion residual cohesion $C_{\rm r}$ $C_{\rm s}$ swelling index C_{t} compression index with respect to net normal stress $C_{\rm w}$ CLOD index $C(\Psi)$ correction function d distance between particles pier shaft diameter d constrained modulus of soil Ddepth of nonexpansive fill D_0 void ratio e base of natural logarithm, 2.71828 einitial void ratio e_0 Young's modulus of the soil E_{s} E_{Δ} Young's modulus of the soil in units of bars f lateral restraint factor

 $f_{\rm s}$ anchorage skin friction

 $f_{\rm u}$ uplift skin friction

 $F_{\rm t}$ nodal force tangent to pier

 $F_{\rm t}$ maximum interior tensile force

g gravitational acceleration

 $G_{\rm s}$ specific gravity of solids

h pressure head

 $h_{\rm d}$ displacement pressure head

 $h_{\rm m}$ matric suction head

 $h_{\rm o}$ osmotic pressure head

H thickness of a layer of soil

 ΔH change in thickness of that layer due to heave

 $H_{\rm t}$ total hydraulic head

i hydraulic head gradient

 $I_{\rm pt}$ instability index

 I_{ss} shrink swell index

k spring constant in APEX

 $K_{\rm a}$ active earth pressure

K(h) coefficient of unsaturated hydraulic conductivity

 K_0 coefficient of earth pressure at rest

 $K_{\rm p}$ passive earth pressure

 $K_{\rm s}$ coefficient of permeability

L length of pier

 $L_{\rm regd}$ required pier length

LL liquid limit

 ΔL / linear strain relative to dry dimensions

 $\Delta L_{\rm D}$ molality mfactor for swelling pressure correlation mporosity of the soil \boldsymbol{n} active earth pressure $p_{\rm a}$ рF unit for soil suction equivalent fluid pressure p_0 passive earth pressure p_{p} P partial pressure of pore water vapor load per linear dimension P P footing load relative humidity P/P_0 $P_{\rm dl}$ dead load on footing PIplasticity index plastic limit PI. P_{max} maximum tensile force saturation pressure of water vapor over a flat P_0 surface of pure water at the same temperature total load due to earth pressure P_0 flow rate of water \boldsymbol{q} distributed load \boldsymbol{q} allowable bearing pressure q_{a} unconfined compressive strength q_{11} mean rate of infiltration at the ground surface $q_{\rm m}$ radius r r^2 correlation coefficient sources or sinks of water $r_{\rm w}$

R universal gas constant

R resistance force

RF risk factor

 $RF_{\rm w}$ weighted risk factor

 $R_{\rm p}$ pullout capacity of helical bearing plate

s coefficient for load effect on heave

S degree of saturation

%SP percent of swelling pressure that is applied by the

total applied stress on the soil

T absolute temperature

 $T_{\rm s}$ surface tension

*u*_a pore air pressure

 $(u_a)_d$ displacement air entry pressure

 $u_{\rm w}$ pore water pressure

U total uplift force

 $U_{\rm t}$ nodal displacement tangent to pier

V molar volume of a solution

V total volume of soil

 $\Delta V/V$ volumetric strain

 $V_{
m w}$ volume of water in an element of soil

w gravimetric water content

 $w_{\rm aev}$ air entry gravimetric water content

 $w_{\rm u}$ gravimetric water content corresponding to a

suction of 1 kPa

 $w_{\rm e}$ weight of sample at equilibrium

 $w_{\rm s}$ weight of oven-dry sample

 y_{max} differential soil movement

$y_{\rm s}$	net ground surface movement
Z	depth
\boldsymbol{Z}	elevation head
\boldsymbol{Z}	soil layer thickness
$z_{\rm A}$	depth of active zone
$Z_{ m AD}$	depth of design active zone
$Z_{\rm p}$	depth of potential heave
$Z_{ m S}$	zone of seasonal moisture fluctuation
$Z_{ m W}$	zone (depth) of wetting
$z_{ m wt}$	height above the water table
Δz	thickness of soil layer
α	compressibility factor
α	soil to pier adhesion factor
α	drainage slope
α_1	coefficient of uplift between the pier and the soil
α_2	coefficient of anchorage between the pier and the soil
β	contact angle with tube
β	reduction factor for expansive earth pressure
γ	unit weight of soil
γ_{d}	dry density of soil
γ_{sat}	saturated unit weight of soil
γ_{t}	total unit weight of soil
γ_{W}	unit weight of water
$\gamma_{\psi o}$	osmotic suction volumetric compression index
γ_{σ}	mean principal stress volumetric compression index

matric suction volumetric compression index $\gamma_{\psi m}$ გ interface friction angle δ_{max} differential heave strain ε isotropic swelling strain $\varepsilon_{\rm iso}$ strain $\varepsilon_{\rm s}$ shrinking strain $\epsilon_{
m sn}$ swelling strain $\varepsilon_{\rm SW}$ total range of strain ϵ_{T} percent swell $\varepsilon_{\rm s}\%$ normalized percent swell $\varepsilon_{\rm S}\%{\rm N}$ percent swell measured from a sample inundated at ε_{s%vo} the overburden stress in the consolidation-swell test. A volumetric water content. volumetric water content above the wetting front θ_{f} θ_r residual volumetric water content $\theta_{\rm s}$ saturated volumetric water content pore size distribution index λ number of ions from one molecule of salt υ Poisson's ratio υ heave ρ differential movement Δρ maximum heave ρ_{max} free-field heave ρ_0 pier heave $\rho_{\rm p}$

solute mass/density

 $\rho_{\rm s}$

```
total heave
\rho_{ult}
          total stress, normal stress
\sigma =
(\sigma' +
u_{\rm w})
\sigma' =
          effective stress
(σ -
u_{\rm w})
\sigma'' =
          net normal stress
(σ -
u_{\rm a})
\sigma''_{cs}
          consolidation-swell swelling pressure
\sigma''_{cv}
          constant volume swelling pressure
\sigma''_{cvN}
          reduced swelling pressure
          external stress
\sigma_{ext}
\sigma''_{\rm f}
          final vertical stress
\sigma''_{\rm h}
          lateral stress
          inundation stress
\sigma_{i}^{"}
          internal stress between particles
\sigma_{int}
\Delta \sigma_{v}^{"}
          increment of applied stress
\sigma_{vo}, \sigma'_{vo} vertical overburden stress in terms of total,
          effective, or net normal stress
\sigma''_{vo}
          shear stress
τ
          osmotic coefficient
φ
          angle of internal friction
φ
          peak angle of internal friction
\varphi_{\alpha}
          residual angle of internal friction
\phi_r
          chi parameter
χ
          total suction
Ψ
```

 $\psi_{ae} \quad \text{ air entry soil suction }$

 $\psi_m = \quad matric \ suction$

 $(u_a -$

u_w)

 ψ_o osmotic suction

 ψ_r residual suction

List of Abbreviations

APEX analysis of piers in expansive soils

ANN artificial neural network

CEAc = CEC/clay cation exchange capacity activity

content

CEC cation exchange capacity

CNS cohesive non-swelling

COLE coefficient of linear extensibility

CS consolidation-swell constant volume

DTA differential thermal analysis

EI expansion index

ET evapotranspiration

FSI free swell index

LE linear extensibility

LMO lime modification optimum

PVC potential volume change

PVC polyvinyl chloride

PVR potential vertical rise

SAMC standard absorption moisture

content

SI shrinkage index SL shrinkage limit

SSA specific surface area

SWCC soil water characteristic curve

SWCR soil water characteristic

relationship

TP total potassium

UV ultraviolet

XRD X-ray diffraction