

Lecture Notes in Mechanical Engineering

Joseph Mathew · C. W. Lim · Lin Ma
Don Sands · Michael E. Cholette
Pietro Borghesani *Editors*

Asset Intelligence through Integration and Interoperability and Contemporary Vibration Engineering Technologies

Proceedings of the 12th World Congress
on Engineering Asset Management and
the 13th International Conference on
Vibration Engineering and Technology of
Machinery

 Springer

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Preface

WCEAM VETOMAC 2017—from 2 to 4 August 2017, Brisbane, Australia

The 12th World Congress on Engineering Asset Management was held jointly with the 13th Vibration Engineering and Technology of Machinery on 2–4 August 2017 at the Brisbane Convention and Exhibition Centre. It was organised by the Asset Institute and hosted annually by the International Society of Engineering Asset Management (ISEAM).

The event shared two themes: “Asset Intelligence through Integration and Interoperability: From Research to Industry” and “Creative and Novel Research for Contemporary Vibration Engineering Technologies” and attracted 244 delegates from 28 countries.

The offerings were rich and diverse. The Opening Ceremony hosted the following speakers:

- Adjunct Professor Joseph Mathew, Asset Institute CEO and Congress Chair, Queensland University of Technology, Australia.
- Professor Joe Amadi-Echendu, ISEAM Chair, University of Pretoria, South Africa.
- Professor C. W. Lim, City University of Hong Kong, China.
- Professor John Bell on behalf of Prof. Arun Sharma, Deputy Vice Chancellor, Queensland University of Technology.
- The Opening Address by the Lord Mayor Graham Quirk, Lord Mayor of Brisbane, Australia.
- The Opening Address by the Platinum Sponsor, Mr. Darren Covington of Mainpac who spoke on, “Delivering Operational Effectiveness in Asset-Intensive Industries through Asset Intelligence”.

The event hosted seven keynotes, a dinner speaker, 14 special sessions on several application and discipline streams, three minicourses, three workshops and three-panel discussions.

Our keynotes this year were:

1. Alan Johnston, MIMOSA and Standards Leadership Council, USA; Jess B. Kozman, Professional Petroleum Data Management (PPDM) Association, Singapore. “Intelligent Integration and Interoperability of Critical Infrastructure and Assets”.
2. Professor Kerry Brown, Edith Cowan University, Australia, “Engineering Asset Management: Understanding the Management Element”.
3. Associate Professor Marco Macchi, Politecnico di Milano, Italy, “The 4th Industry Revolution: Reflecting on the Opportunities, Barriers and Risks for Asset Management”.
4. Professor Romuald Rzadkowski, Airforce Institute of Technology, Poland, “Asset Management Through Life Estimation”.
5. Professor Tang Loon Ching, National University of Singapore, “Systems Resilience: a Unifying Framework and Associated Measures”.
6. Professor Klaus Blache, University of Tennessee, USA, “Technologies and Asset Management: What is really going on in Industry”.
7. Professor J. P. Liyanage, Cluster for Industrial Asset Management, University of Stavanger, Norway, “High Risk Assets Under Uncertain Conditions: Strategic Imperatives and New Initiatives Towards Defensive Solutions in a Rapidly Changing Environment”.

The dinner speaker was Dr. Paul Simshauser, Director General, Department of Energy and Water Supply, Queensland Government, who spoke on “Energy industry—challenges ahead”.

One hundred and fifty management and technical presentations reported on outputs of research and development activities as well as the application of knowledge to industry and government in the practical aspects of the various themes that comprises Engineering Asset Management and Vibration Engineering. These papers and presentations were made available for the personal reference of delegates only via an eProceedings Dropbox repository at the Congress, and not for distribution.

Full papers were further peer-reviewed by two members of the Scientific Committee and this e-book contains the final selection of these papers.

The Congress was augmented with a series of three interactive workshops.

The first of these workshops was entitled “**The Long Future Sustainability for Asset Managers**” in which David Hood and Guy Lane encouraged participants to think about and discuss their individual mindsets and roles as asset managers in influencing and creating a sustainable future for our planet. At the workshop, David remarked that the principle of sustainability is often confined to the financial sense rather than encompassing a more holistic and long-term sense by including environmental sustainability.

Jess Kozman, a Regional Representative of the Professional Petroleum Data Management Association and independent data management practitioner, lead a workshop on the “**Evaluation of data management maturity in relation to**

engineering assets". The workshop took participants through a short exercise in which a range of survey questions was used to establish the data management maturity of the participant's organisations. The workshop was attended by participants from a broad range of industries and backgrounds. Some practical ideas in relation to the application of maturity surveys were given by Jess.

"Fostering the development of Engineering Asset Management Programs at Higher Educational Institutions via Recognition" was the subject of the final workshop. A highly interactive discussion was led by Prof. Joe Amadi-Echendu, Chair of ISEAM. Participants discussed the merits of recognition rather than accreditation of programs, the development and custodianship of the Asset Management Body of Knowledge and made comparisons between Asset Management and the Project Management Institute and their development path.

Three minicourses were conducted on the final day of the Congress. Alan Johnston from MIMOSA led the training session entitled **"The open industrial interoperability ecosystem, a supplier-neutral digital ecosystem, enabling critical infrastructure and industrial asset management"**. Professor Romuald Rzakowski from the Polish Academy of Sciences conducted the course **"Life estimation and exact time of failure of last stage steam turbine blades"**. Dr. Carla Boehl from Curtin University held a training session on **"Mine Autonomous Haul System: Assessing the Impact in Asset Management"**. Each of the minicourses was well attended.

The generous support of our Sponsors is acknowledged. They were our Platinum Sponsor, Mainpac, our Silver Sponsors, Brisbane City Council, Schneider Electric and the Fredon Group. Bronze Sponsors: QUT, Synengco, NMEMS Japan, K2Fly and Redeye, as well as our Exhibitors, Asset Finda, SAVTek, Springer and Request Direct.

Also acknowledged were our partners who assisted us in organising this event and providing some of our key speakers, i.e. MIMOSA, PPDM, the Vibration Institute of India, the IFAC A-MEST Working Group, the 2017 Global Business Challenge and NCCARF, Australia (Fig. 1).

This year ISEAM gave out two awards for the Best Paper Award, one for the management category and the other for the technical category and acknowledged the runner-ups as well. These were:

Management

Winner: "Analysing an Industrial Safety Process through Process Mining: A Case Study"

Authors: Anastasiia Pika, Arthur H. M. ter Hofstede, Robert K. Perrons, (QUT) and Georg Grossmann, Markus Stumptner, (University of South Australia), and Jim Cooley (Origin Energy).

Runner-up: "Semiparametric valuation of heterogenous assets".

Authors: Roar Adland and Sebastian Köhn, Norwegian School of Economics (NHH), Norway.



Fig. 1 Associate Professor Rob Perrons receiving the award from Adjunct Professor Mathew with Prof. Markus Stumptner on left



Fig. 2 Mr. Norihiko Ogura receiving the award from the Congress Chair

Technical

Winner: “Efficient Evaluation of Internal Concrete Damage of Steel Plate-bonded RC Slabs”.

Authors: Norihiko Ogura, CORE Institute of Technology Corp, Hitoshi Yatsumoto, Hanshin Expressway Company Ltd. and Takahiro Nishida and Tomoki Shiotani, Kyoto University (Fig. 2).

Runner-up: Joint Optimization of Preventive Maintenance and Spare Parts Logistics for Multi-echelon Geographically Dispersed System.

Authors: Keren Wang and Dragan Djurdjanovic, University of Texas at Austin, USA.

The 2017 ISEAM Lifetime Achievement Award was presented to Emeritus Prof. Robert B Randall, University of New South Wales, Australia.

ISEAM’s Lifetime Achievement Award recognises and promotes individuals who have made a significant contribution to research, application and practice of a discipline or in engineering asset management over a continued period of time (Fig. 3).

The Congress Chair was especially honoured by ISEAM at the Congress with a Lifetime Achievement Award. He is the 5th recipient of this award since the formation of the society in 2006.

In addition, ISEAM further honoured the Congress Chair with a Pioneer Award in recognition of his foundational contributions to ISEAM as its Founder and Chair



Fig. 3 Professor Randall receiving the Lifetime Achievement Award from the ISEAM Chair



Fig. 4 Professor Mathew receiving the Lifetime Achievement Award from the ISEAM Chair

of the Board in its first 11 years. The awards were presented by the ISEAM Chair, Prof. Joe Amadi-Echendu, University of Pretoria, South Africa.

The event was a resounding success judging from the feedback of the delegates and the program may be used in future WCEAMs' as a template for transnational, trans-sectoral and transdisciplinary knowledge sharing events in asset management going into the future (Fig. 4).

We are grateful for all the voluntary assistance provided by members of the International Scientific Committee, the Organisation Committee and the Conference Secretariat who are all acknowledged in the following sections.

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Congress Chair: Adjunct Professor Joseph Mathew, Asset Institute, Queensland University of Technology, Australia

Co-chairs: Dr. Trudy Curtis, Professional Petroleum Data Managers (PPDM) Association, Canada

Alan Johnston, MIMOSA, USA

Professor C. W. Lim, City University of Hong Kong, Hong Kong

Associate Professor Marco Macchi, Politecnico di Milano, Italy

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 Brisbane, Australia

Joseph Mathew
 C. W. Lim
 Lin Ma
 Don Sands
 Michael E. Cholette
 Pietro Borghesani

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Pengyu Zhu

Predicting the Remaining Life of Timber Bridges



T. Abbott, N. Gamage, S. Setunge and Weena Lokuge

Abstract This paper documents the current state of knowledge relating to the deterioration of timber bridges in Australia. The aim of this research, was to comprehend the present state of knowledge regarding maintenance of timber bridges and address any gap in knowledge. This involved: identifying key defects in timber, investigating the inspection methods utilised to detect these faults and finding the preventive measures used to mitigate bridge deterioration. Enclosed are figures which demonstrate how simple industry practices and procedures implemented by each states' governing authority can reduce these impacts and concludes with an empirical model for predicting the remaining lifespan of a bridge.

1 Introduction

Of the roughly 40,000 bridges in Australia, 27,000 of them are constructed of timber. Most of these are over 50 years old and in a weathered condition [4]. It is obvious (Fig. 1) that the timber bridge stock is depleting irrespective of the efforts in the rehabilitation processes [1]. From government documents and first hand correspondence with engineers in the industry it has been discovered that a lot of these bridges have been replaced by steel elements. The majority of remaining

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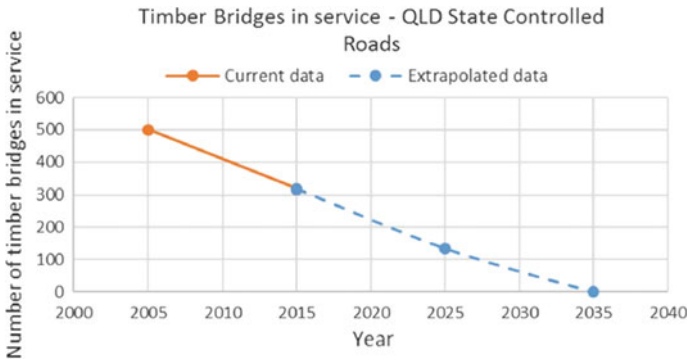


Fig. 1 Timber bridge stock in Queensland

timber bridges in Victoria, are of the girder deck system type and located on forest walk trails and tourist roads or are local roads which are controlled by municipalities [5]. There are a number of considerations which affect how a bridge will perform throughout its service life. The design load, environmental factors, type of timber and size of members used all contribute to which known defects each bridge element will be most susceptible to and the best method of detection [3]. With the knowledge contained in this report and previous history a matrix has been developed highlighting these key aspects. Inspectors can use this for their reference to check typical symptoms of decay and take appropriate actions.

The major defects that all Australian timber bridges are susceptible to are splitting and rot due to fungal attack. If the bridge is used as a water crossing, marine borers and flood are other possible factors. Termite infestation is not as predominate in southern states, although reports do suggest that in warmer states such as Queensland, this may be more of threat to timber structures. Inspectors have reported that it is more likely to see timber shrink and be pulled off their fasteners than it is to see the iron nails corrode, thus this form of deterioration is not very significant. The stringers of a bridge are subjected to substantial load distributed from the superstructure and may undergo excessive deflection over time in a phenomenon known as creep. Inspection techniques are divided into destructive and non-destructive. The most common and reliable destructive tool involves drilling into a timber member to grasp its interior condition. Currently in Australia, state governing bodies are responsible for the upkeep of their timber bridges intrust their skilled engineers to visually inspect the structures and decide which course of action to take.

2 Industry Practice

The following is a paraphrased excerpt from a conversation with two structural engineers who have decades of experience in inspecting timber bridges. Their knowledge outlines the current Victorian practice and highlights their conclusions

about deterioration they gathered from practical inspections in the field. The governing authority employed a number of cost-effective methods of preventing damage to timber elements including a range of epoxies and paints to protect the outside surface from moisture and debris build up which could lead to decay. The implementation of anti-split bolts and washers were installed on the piles to reduce the impact deep checks would have to the column. Petroleum jelly was also used as a water proof barrier. These measures are implemented on a case by case manner as recommended by the inspecting engineer, without any formal standard to follow.

There are a number of major factors to look for when attempting to estimate the remaining lifespan of a bridge. It is imperative to inspect the condition of every elements' surface, checking for moisture and debris accumulation which could lead to decay and risk in case of fire. It is important to check for splits and cracks in corbels and cross beams. There is no cause for alarm if sapwood is seen rotting as this always rots first and doesn't contribute to section capacity. It is humidity, soil presence and moisture govern how a timber will deteriorate. Timber bridges were abundant in Victoria during the 1940s due to availability of wood. The mindset back then was to aim for fifty years life from the structure through inexpensive, low level maintenance.

2.1 Treatment Methods

Treatment methods are often referred to as maintenance because of the existing proactive understanding in the industry. The ethic is that it is more beneficial to increase the lifespan of a bridge and its components through regular inspection and maintenance than to replace decrepit members on a need to basis. Frequent upkeep and inspection can report on the derogation rate and suggest recommendation for planned works in the future. The method of replacing members once they have decayed beyond repair is a costly exercise and by the time the deterioration is detected the operational effectiveness of the structure can be compromised, resulting in serviceability failure and a risk to safety [2].

The maintenance methods can be divided into three branches which are distinguished by the cost, timeframe and level of work performed. The simplest and cheapest form of treatment is Routine Maintenance/Preventative Treatment. This consists of mainly minor reactive works which are anticipated and allocated for in the budget and planned on a short term basis, usually about two weeks or less [2]. This commonly includes controlling factors which provide favourable conditions for decay such as moisture content. Periodic Maintenance/Early Remedial Treatments are carried out at regular intervals of longer than one year. These are designed to fix problems associated with early stage defects, such as rot. It is undertaken on a proactive rather than reactive basis [2]. Examples include baiting systems to deter termites, the installation of pile jackets and more rigorous application of sealants. Specific works/major maintenance occurs when decay is so advanced that the member simply does not have the structural strength to support

the loads acting on it. Expensive replacement type improvement maintenance or rehabilitation maintenance is used when a member is so decayed regular treatments will not repair its structural integrity. These include one-off repairs, refurbishment and upgrade works to retain the bridge as close as possible to its original condition.

3 Defect Treatment Protocol

Deterioration is a cause and effect process. Each type requires certain factors to manifest. These can be detected with different methods and categorised into routine maintenance mitigation, periodic maintenance and major works or rehabilitation maintenance. The following (Fig. 2) is an amended defect treatment matrix adopted from the Queensland Department of Transport and Main Road's Timber Bridge Maintenance Manual [6].

Matrix shown in Fig. 2 is a tabulated illustration condensing all the information found in the maintenance manuals. This figure details the type of defects, how they are identified and how they are mitigated through maintenance techniques. It provides a snapshot to inspectors at what to look for, how to detect and how to treat various forms of deterioration. The matrix does not give an indication to how long a bridge may last until it is unsafe, it only gives protocols to follow at the time of inspection. Therefore current practice shows that there is no concise methodical approach to quickly ascertain the required information about how to mitigate or predict the rate of timber bridge deterioration, which is reinforced by other researchers [4]. Equipped with this knowledge, a procedure collaborating all the necessary data has been developed to address the gap in current knowledge. It is anticipated that these tables can be used by inspectors to empirically rate and score the level of deterioration of a bridge.

Taking a closer look at stringers the usual causes for deterioration are: pipe rot, splitting and termite infiltration. Depending on the design and member dimensions, excessive deflection may also prematurely weaken the member. Figure 3 shows a worst case scenario for the prognosis of a stringer in a poorly fabricated and executed maintenance regime. Using the stringer member as an example, the flowchart shown above displays the major contributors to the element's structural demise as well as indicating the cause of each defect. The exterior boxes describe the factors present which cause the four most common types of decay to the stringer element. Interestingly, termite infestation occurs when rot breaks down the wood fibers allowing the organisms to enter the dark cavities, which occurs when timber shrinks releasing water attracting fungal spores to grow and start decaying the wood. Therefore, for termites to be present, the wood must first have been weakened by fungal rot to improve the conditions for termites to thrive. This shows a more appealing outcome for the condition of a bridge. With an effective mitigation

Defect	Caused by	Inspection Technique	Treatment Category	Treatment Action
Severe Splitting	Internal tensile stresses built up due to drying	Visual- check ends, occurs along the grain, be weary of water presence	Specific works/ replacement	Replace timber girder
Significant splitting			Periodic/ early remedial	Install anti-split bolts for mitigation
Fasteners corroded	Rust (rain, oxygen)	Visual, half-cell voltage	Specific works/ replacement	Replace or install bolts
Girder to cap beam bolts loose due to shrinkage	Uneven drying of timber member	Visual, physically testing strength	Routine/ preventative	Tighten existing bolts
Termite infestation	Existing decay, no light, humid moist atmosphere	Acoustic, coring,	Periodic/ early-remedial	Eradicate nest
Localised termite presence		Acoustic, visual, chipping	Routine/preventative	Inject poison into member
Crushing at end of span	Excessive shear force	Visual deformation present	Periodic/ early-remedial	Add supporting member
Light to medium Fire damage	Flammable environment	Visual	Periodic/ early-remedial	Apply chemical preservative
Fungal decay (up to 30% diameter loss at mid-span and 20% at ends)	Supply of food, moist atmosphere, suitable temperature and oxygen	Resistograph, coring, visual (if decay is external)	Routine/ preventative	Apply chemical preservative to slow decay rate
Fungal rot (up to 70% diameter loss at mid-span and 50% at ends)		Stress wave timing, chipping, coring, acoustic	Specific Works/ major maintenance or replacement	Replace timber stringer or install steel girder
Any defect causing more than 70% loss of section regardless of cause	Failure to implement an effective maintenance program	Measuring residual strength using more accurate and intricate methods (coring and stress wave timing)	Specific Works/major maintenance or replacement	Seek advice from structures division to decide whether to demolish or attempt to restore

Fig. 2 Defect treatment matrix

protocol in place, this bridge will outlast its above counterpart. One noteworthy remark is that this flowchart only shows the major causes affecting the deterioration rate. Other less common factors such as flooding or fire may contribute to the ultimate design life.

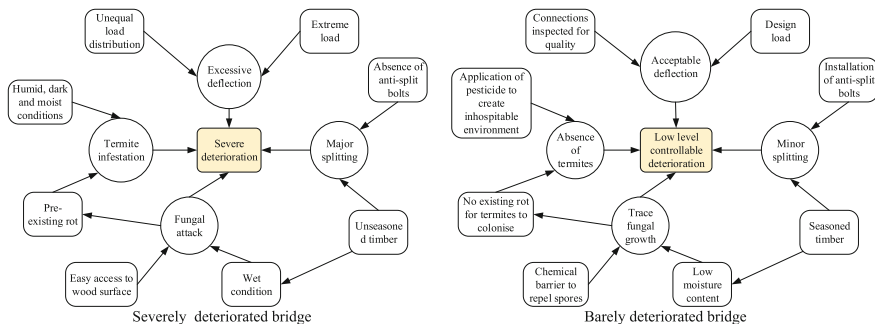


Fig. 3 Flowchart of bridge, highlighting the causes and factors contributing to respective states

4 Bridge Deterioration Prediction Model

After researching the current state of knowledge and delving into the information provided in the states' maintenance manuals, it became apparent that there was no documented way to predict the remaining service life of a bridge based on environmental factors which manifested the identifiable defects within it. Within this section is a prototype of an empirical model to produce an educated estimate of the amount of time, in years, until the overall condition of a timber bridge surveyed reduces to condition state 4.

This model depends on the type of decay present to predict the serviceability of the bridge. Therefore the first step is identifying the modes of deterioration. The five main constituents are fungus, splitting, termites, corrosion of fasteners and marine borers, which affect the three elements differently. Below are clauses to be considered when applying this model.

- Clause (a) Termites are only considered for bridges in warm climates. For the purpose of this model, “warm climates” include Queensland, Western Australia and the Northern Territory regions.
- Clause (b) Marine Borers are only considered for bridges where there is significant water presence that has prolonged contact with timber surfaces, such as rivers and creeks.
- Clause (c) Each form of decay affects each bridge element to various degrees of significance. Bridge Stringers will be susceptible to: Fungal attack, termite attack (if located in region defined in clause a) and splitting. Bridge Piles will be susceptible to: Fungal attack, termite attack (if located in region defined in clause a), splitting and marine borers (if situation satisfies clause b). Bridge Corbels will be susceptible to: Fungal attack, termite attack (if located in region defined in clause a), splitting and fastener corrosion.

- Clause (d) any defect which causes more than 50% loss of section (beyond the parameters of Condition state 4) is said to be unsafe as it has failed and requires immediate replacement of the member.

4.1 Limitations

This prototype focuses on three critical structural members that make up the sub-structure of the bridge. These are the stringers, corbels and piles. For this model to be implemented accurately, the inspector has to comment on the presence of each type of decay prone to a particular member. That is, they should not conclude a condition state based purely on one mode of deterioration, but should endeavour to observe other forms applicable to the element which may accelerate decay. The values assigned to each condition state are purely estimated with no laboratory testing to authenticate the figures delegated. It is also possible for one element to have a different condition state to another element of the same type. When this happens, it is up to the inspector’s discretion to implement a value which best fits the description for the entire number of members of that type. For simplicity of the model and due to resource restrictions, it is assumed that degradation of a bridge is linear in nature.

The prediction model is based on the observations made during inspections. However it can accommodate quantitative observations or measurements as well using structural health monitoring devices. Further development of the model can be done based on differentiation between the wood species.

4.2 Model

The following tabulates all of the steps required to classify the condition of a bridge via its empirical score (S) and therefore compute its remaining lifespan. This model encompasses all information which has been gathered from this research and will need to be referred to acquire figures. Table 1 demonstrated the allocated distribution for each decay type for each element. The element in question and number of

Table 1 Deterioration weighted splits

Decay type	Weight as a percentage			
	Stringer/pile	Pile	Stringer	Corbel
Fungus	60	45	50	40
Splitting	40	30	30	30
Termite	–	10	20	15
Marine Borer	–	15	–	–
Corrosion	–	–	–	15
Element	Stringer/pile	Pile	Stringer	Corbel

applicable methods of degeneration determine how much each type of decay contributes to the total amount of deterioration; see procedure clauses a, b and c.

As shown in Table 2, points are assigned for each condition state. Using the condition states listed earlier in this report the inspector can match the description of the bridge element to the most suitable condition state. This process will have to be repeated for each element (stringer, corbel and pile). See also procedure clause d. There may not be four decay types for every element based on environment.

The total weight for each element thus found is inputted into the cell under the respective element. The average is then calculated. Before the final score is given the entire bridge can be subjected to environmental factors which may adjust its overall score (Table 3).

Sum the values of both columns that are applicable based on inspector's general observations. Table 4 provides a numerical and descriptive indication of a bridge's integrity. This result can be compared to future inspections to summarise a trend and determine the rate of deterioration.

Following equation can be used to determine the time the bridge has until it reaches condition state 4. The equation is based on the scores for two extreme ends of the spectrum in Table 4. As shown in Table 2, a bridge in condition four still receives points and allocated a score of 3 in Eq. 1.

$$v = \frac{S - 3}{10 - S} * T \quad (1)$$

Table 2 Points assigned for each condition state

Condition state	1	2	3	4
Points assigned	9	6	4	2

Table 3 General observations modification adjustment

Discolouration	-0.1	Fresh, new coating	+0.1
Loss of fill in abutments	-0.1	Deck well maintained	+0.1
Deck cracking	-0.2	Sapwood still intact	+0.2
Undersize members	-0.3	No visible deflection under load	+0.2
Visible decay in other bridge members	-0.3	No debris on any bridge component	+0.3

Table 4 Timber bridge classes

Score (S)	0-3	3.1-5	5.1-7.5	7.6-10
Classification	Poor	Fair	Good	Excellent

v = Remaining time in years until condition state 4 is reached; S = Score calculated from Table 6; and T = Age of bridge when inspection was conducted, in years. Inspectors would then refer to the defect treatment matrix for maintenance options and recommended mitigation techniques to preserve existing infrastructure to prolong the service life of the bridge.

4.3 Demonstration of the Model

20 year old two span simple beam wooden bridge constructed from seasoned F grade timber is selected to demonstrate the developed model. Bridge deck consists of timber planks installed transversely with bitumen sealed to support loads up to 20 t. Mid-span piles are submerged up to half the length of the pile in a river. Considerable discolouration and decay (~20%) was observed throughout all elements of the superstructure and substructure. Deck is still within serviceability limit state and deck and top of headstocks were littered with debris. Pile is in a more serious decayed state due to multiple types of decay present.

Tables 5 and 6 demonstrates the calculations for the final score and using Eq. 1 the remaining life of this bridge can be calculated as 13.57 years $[(5.83-3)/(10-5.83)]$. Treatment matrix such as the one shown in Fig. 2 can be used to determine the maintenance process for this bridge.

Table 5 Element designation and point matrix

Element	Decay	Description	Condition	Points	Weight
Stringer	Fungus	~20% pipe rot	2	6	3
	Splitting	Minor-medium splitting	2	5	1.5
	Termite	Minor presence	1	9	1.8
Total				20	6.3
Corbel	Fungus	~20% pipe rot	2	6	2.4
	Splitting	Minor splitting	2	6	1.8
	Termite	Minor presence	1	9	1.35
	Corrosion	Fasteners slightly loose	2	6	0.9
Total				27	6.45
Pile	Fungus	~30% pipe rot	3	5	2.25
	Splitting	Moderate splitting	3	5	1.5
	Termite	None	1	10	1
	Marine	Significant activity	3	4	0.6
Total				24	5.35

Table 6 Computation of final score (S)

Stringer weight	Corbel weight	Pile weight	Average weight
6.3	6.45	5.35	6.03
General observations modifications	No visible deflection +0.2 Visible decay -0.3 Discolorations -0.1	Final score (S) classification	5.83 Good

5 Conclusions

This research has been conducted through a highly theoretical and analytical approach. From the beginning the purpose of this research has been to report on the characteristics of timber as it degrades throughout its lifespan and what current authorities are doing to mitigate damage and maintain their infrastructure. The bridge deterioration prediction model has been completed which was the key deliverable in this paper. This model should satisfactorily address the gap in knowledge regarding this subject matter. Further research is necessary to access and validate the accuracy of the model. With more data and analysis of how deterioration develops, better time estimates can be recorded and the model can be recalibrated to improve its application throughout Australia’s timber bridges. The best way to validate this research is to apply it to previously documented bridges or to conduct a study to observe and record in-service bridges’ deterioration rates to check if they follow the predicted relationship.

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